4 Turkey Hill Road Newtown, CT 06470 Tel (203) 270-4300 Fax (203) 426-9968

Fred Hurley,

Director



TOWN OF NEWTOWN WATER AND SEWER AUTHORITY

Marianne Brown,
Chairman
Richard Zang
Louis Carbone
George Hill
Alan Shepard
Eugene Vetrano
Carl Zencey

THESE MINUTES ARE SUBJECT TO APPROVAL BY THE WATER AND SEWER AUTHORITY

The Water and Sewer Authority a regular meeting on June 8, 2017 at the Wasterwater Treatment Plant, 24 Commerce Road, Newtown, CT. Dick Zang called the meeting to order at 7:00pm.

Present: Lou Carbone, George Hill, Dick Zang, Alan Shepard, Carl Zencey

Absent: Gene Vetrano, Marianne Brown

Also Present: Director of Public Work Fred Hurley and Julio Segarra of Suez Water Environmental

Services

Public Participation – NONE

Approval of the Minutes – George Hill moved to approved the minutes of the May 11, 2017.

Corrections: 3 members of the public and 1 member of the press were present, motion for 12 Church Hill is resynced, the information used was for 12 Queen Street. Minutes unanimously approved with corrections.

UNFINISHED BUSINESS

Hawleyville Sewer Extension — Fred Hurley reported that there is no improvement with Centerplan. They are essentially out of the project. The final punch list will be completed by Public Works and Suez. When the water line goes in for the multi housing project, WSA will share the cost of paving. The WSA has been advised by the Town Attorney not to pay the vendors from the retainage. Vendors will be referred to the bonding company for payment.

NEW BUSINESS

Set Sewer Benefit Assessment for 12 Church Hill Road – a request for the benefit assessment has been made.

Sandy Hook Pump Station and Available System Capacity – They went over the preliminary analyses from Fuss and O'Neil (Attachment A). The piping is ok but will be cleaned out. The capacity of the pipes is fine but the capacity of the pumps needs to be addressed.

Fred Hurley provided a Preliminary Analysis of Available Sewer Capacity (Attachment B). In that analysis he is trying to anticipate projects that are coming in. The WSA does not have the projects before them for The Boulevard or 79 Church Hill Road. They were told that their projects need to go through P&Z and have an approved project. Alan Shepard suggested a joint meeting with P&Z to make sure both boards are aware of the process. For projects that there is a sewer extension, P&Z would have to request an opinion of the WSA before approval of any application by P&Z.

Committee Reports – I&I; An aggressive I&I program should be able to return 10%-15% of the system capacity. Julio Segarra discussed several procedures to clear lines using crushed ice and improved air relief values. Well development; Three months ago well 3 was shut down but is now back in operation. SCADA; There were two problems. One was two imbedded databases that was discovered programmed into the RTU. Removal of one solved the problem. Second was several communication problems solved by antennae redirection.

Report by Suez Water Environmental Services – Monthly report (Attachment C).

Report by Public Director – Fred Hurley reported that he spoke with Mike Burton regarding development on Washington Avenue. They have gotten good news from the bank and it looks like they are going to move forward.

Having no further business, the meeting was adjourned at 8:20pm.

Arlene Miles, Clerk





MEMORANDUM

TO:

Mr. Fred Hurley

Newtown DPW Director

FROM:

Fereshteh Doost, P.E., Kurt A. Mailman, P.E.

DATE:

April 25, 2017

RE:

Cursory Sandy Hook Pump Station Capacity Study

The Newtown Water and Sewer Authority (WSA) solicited the services of Fuss & O'Neill to cursorily evaluate the Sandy Hook Pump Station for increased wastewater flows due to two proposed developments and a sewer extension within the sewershed. As part of this evaluation, the following tasks have been performed:

- Average and peak hour flows have been calculated for the proposed developments and sewer extension
- Existing pump data for the past two years has been reviewed
- System hydraulics with potential increased flows have been calculated
- Impacts of potential increased flows on existing Sandy Hook (Glen Road) Pump Station (pumps, electrical, generator, etc.) have been evaluated
- Impacts of potential increased flows on the existing force main downstream of the pump station have been evaluated

It is noted that this cursory evaluation was limited to a desktop study of the pump station records and potential wastewater flow increases, and did not include a physical evaluation of the pump station or collection system.

Flow Calculations:

For the purposes of projecting flows from the proposed developments (179 Church Hill Rd., River Walk) and sewer extension to an existing commercial building that would connect to the Sandy Hook Pump Station, average and peak hourly wastewater flows were calculated based on the Newtown Water and Sewer Authority Sewer Use Regulations dated October 29, 2015. Details regarding the expected flows from each source are provided below:

- The flow for the proposed 225 units at 79 Church Hill Road was estimated based on multifamily housing units (148 gpd/unit, including I/I).
- The flow for the proposed 65 units at River Walk Road was estimated based on multifamily housing units (148 gpd/unit, including I/I).
- A design flow of 10,000 gallon per day (gpd) was assumed for the proposed fabric fire hose commercial building (based on meeting with DPW).

\\private\dfs\ProjectData\P1992\92248\A53-Sandy Hook PS Evaluation\Deliverables\Report\20170420_Sandy Hook Pump Station Capacity Memorandum.docx



Sandy Hook Pump Station Capacity- Ms. Marianne Brown April 25, 2017 Page 2 of 3

According to TR-16 (Guides for the Design of Wastewater Treatment Works 2011 edition, as revised in 2016) by the New England Interstate Water Pollution Control Commission, sanitary sewers should be designed on a peak hourly design flow basis. The ratio of extreme peak hourly flow to average daily flow is typically 4.0-5.6 times the average daily flow for wastewater generation, pursuant to Table 2-1 of TR-16 (Exhibit 1). To obtain a peak hour, a representative peaking factor of 4.5 was multiplied by the average daily wastewater flows.

The existing pump data for the past two years has been reviewed (See Exhibit 2). The 2016 data was used for the hydraulic calculations. See Table -1 below for existing and proposed wastewater flows to the pump station.

Table 1 – Existing and Proposed Wastewater Flow

	Average Do	ily Flow	Peak Hourly Flow
	GPD	GPM	GPM
Existing Flow (2016)	58,705	41	185
79 Church Hill Road	33,300	23	104
River Walk	9,620	7	30
Commercial Building	10,000	6.9	31
Total	111,625	78	350

The original flow (calculated in 1994) for the Sandy Hook Pump Station was 98 gallons per minute (gpm) and pump design capacity was 200 gpm. One of the existing pumps is currently pumping below its original rated capacity and is not meeting cleansing velocity requirement for the force main. The capacities of Pump #1 and Pump #2 are 215 gpm and 180 gpm, respectively (as presented by the SUEZ Plant Manager). Adding the calculated flows for 79 Church Hill Road, River Walk Apartments, and a new fabric fire hose commercial building to the existing peak flow of 185 gpm (in 2016) would increase the design flow to 350 gpm., which cannot be supported by the existing pumps (See Exhibit 3 for existing pump curve and Table-2 of Exhibit 4 for pump station capacity). The combined capacities of the existing pumps are less than 240 gpm. (Pump station is not designed for pumps to operate in parallel under normal operations).

Hydraulic Calculations:

The Hazen-Williams equation is used to determine the head loss through pressurized force mains. Hazen-Williams coefficients of 120 were used for old ductile iron (DI) pipes. The total dynamic head



Sandy Hook Pump Station Capacity- Ms. Marianne Brown April 25, 2017 Page 3 of 3

(TDH) for the existing plus proposed flows (350 gpm) was calculated to be 200 feet and is presented in **Table -3** of **Exhibit 4**.

Cursory Pump Station Evaluation:

Data indicates that the Sandy Hook Pump Station is currently at its design capacity and existing pumps cannot handle greater flow, particularly due to the reduced pumping capacity of Pump #2. The pump station is currently reportedly over-cycling (exceeding the recommended number of pump starts per hour) and would not be capable of pumping the proposed additional flows without surcharging. In March 2017 repairs reportedly occurred to fix a clogged lateral at the High School, upstream of the Sandy Hook Pump Station. Once the repairs were complete, the flow rushed into the pump station. Even with both pumps running in parallel, the pumps could not handle the flow of +/- 250 gpm as estimated by the SUEZ Plant Manager.

Force Main Evaluation:

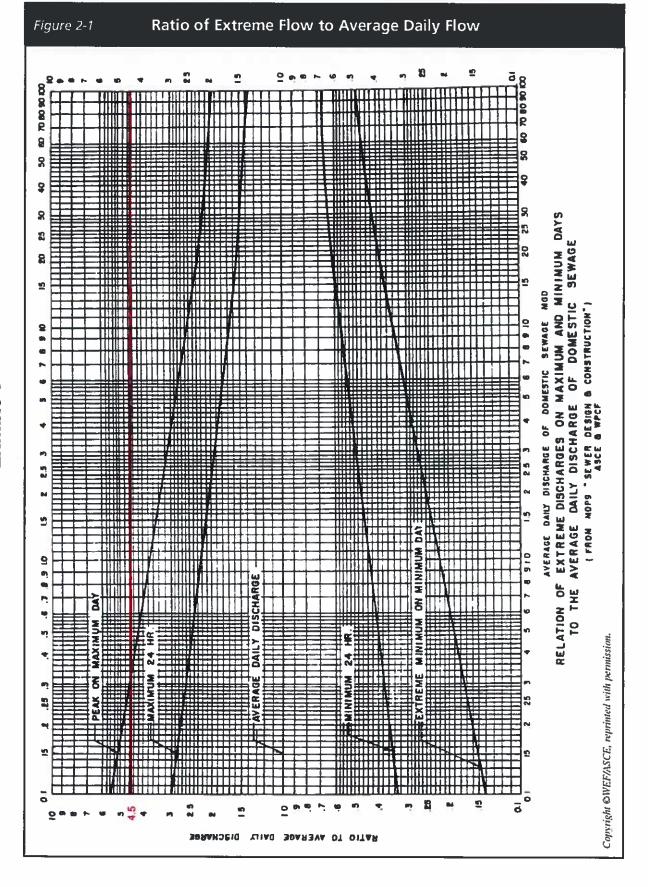
Flows from the pump station are pumped through a 6-inch force main (3,120 ft.) into a termination manhole (SMH #51) which is located near the I-84 interstate at Church Hill Road. With an existing pumping rate of 180 gpm from Pump #2, the force main line velocity is 2.04 feet per second (ft./s). The existing velocity does not provide the minimum 3.0 ft./s cleansing velocity required for the force main, pursuant to TR-16. However, the collection system downstream of the Sandy Hook Pump Station is capable of transporting the proposed design flows. The new pumps will be sized for the proposed pumping rate to ensure velocities in the force main are greater than 3.0 ft./s in order to provide enough scour to keep the force main clean. Utilizing the proposed design flow of 350 gpm, the force main line velocity would be 3.97 ft./s, which is above the minimum cleansing velocity of 3.0 ft./s (**Table 3** of **Exhibit 4**).

Minimum Recommended Pump Station Improvements to Accommodate Proposed Additional Flow:

At a minimum, the two existing 25 HP submersible pumps would need to be replaced with larger 50 HP pumps. Exhibit 5 presents the proposed pump. In addition, the current start-stop controls should to be upgraded to variable frequency drives (VFDs) or soft starts-stops to reduce the increased inrush current associated with the larger pump horsepower and reduce electrical costs. Additional improvements would likely be required due to the age and condition of the existing pump station including replacement of the existing check valves, electrical panel, electrical service, and standby exterior mounted generator with sound attenuation. Cleaning the force main to remove any sediment, debris (perhaps via ice slurry) and cleaning/exercising the air relief/vacuum valve is also recommended.

In order to determine the full extent of improvements and costs required to upgrade the pump station to accommodate the proposed additional wastewater flows additional design, including a complete pump station evaluation, would be required.

Sandy Hook Pump Station Capacity Study Exhibit 1



Sandy Hook Pump Station Capacity Study Exhibit 2

2015

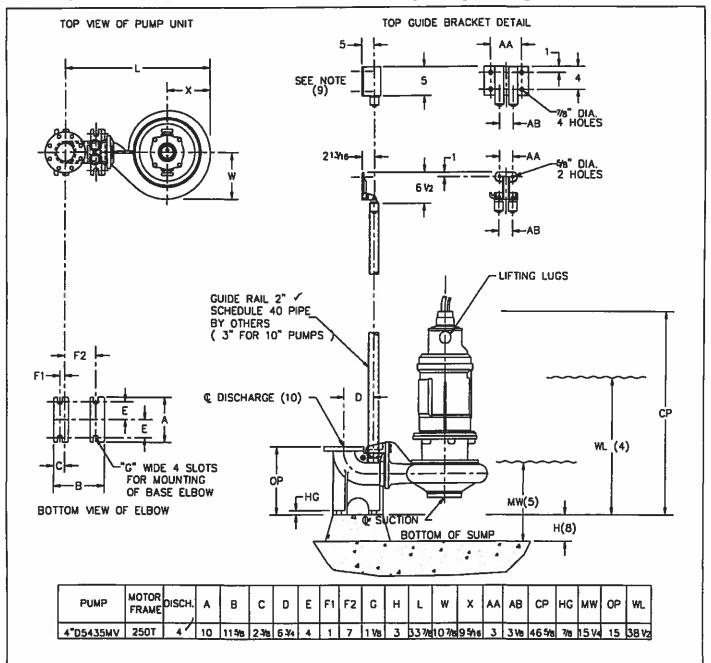
2016

CT NEWTOWN WWTP (NEWTOWN, CT)

		Sandy	Sandy Hook PS Flow	Flow					Sandy	Sandy Hook PS Flow	Flow		
Date	Total	Average	Maximı	Maximum Day	Minimum Day	ım Day	Date	Total	Average	Maxim	Maximum Day	Minimum Day	ım Day
Jan '15	1,687,088	54,422	01/13/15	61,631	01/16/15	50,247	Jan '16	1,631,717	52,636	91/08/10	56,375	91/97/10	48,679
Feb '15	1,496,773	53,456	02/27/15	59,247	02/01/15	51,662	Feb '16	1,605,347	55,357	07/97/16	63,428	02/12/16	47,388
Mar '15	1,854,989	59,838	03/13/15	69,984	03/06/15	54,446	Mar '16	1,821,653	58,763	03/28/16	81,272	03/24/16	50,240
Apr '15	1,804,089	60,136	04/02/15	67,523	04/14/15	49,193	Apr.16	1,629,917	54,331	04/08/16	57,068	04/12/16	47,934
May 15	1,928,273	62,202	05/29/15	81,341	05/12/15	54,173	May '16	1,800,627	58,085	91/16/50	105,430	05/20/16	49,055
Jun '15	1,722,131	57,404	06/02/15	83,533	06/30/15	39,934	91, unf	1,801,566	60,052	91/10/90	105,430	06/28/16	51,834
51, Inf	1,495,997	48,258	07/07/15	55,752	07/01/15	39,934	91. Jnf	1,738,151	56,069	91/61//0	63,918	07/05/16	52,433
Aug '15	1,467,234	47,330	08/28/15	51,467	08/14/15	44,851	Aug '16	2,234,772	72,089	08/30/16	266,270	91/10/80	53,634
Sep '15	1,552,311	51,744	09/18/15	55,396	09/08/15	47,368	Sep '16	2,012,213	67,074	91/10/60	266,270	91/06/60	56,274
Oct '15	1,638,887	52,867	10/02/15	29,960	10/16/15	48,201	Oct '16	1,812,391	58,464	10/25/16	62,493	10/18/16	56,178
Nov '15	1,486,089	49,536	11/10/15	54,576	11/24/15	42,445	91, voN	1,754,460	58,482	11/11/16	64,061	91/62/11	51,534
Dec '15	1,562,967	50,418	12/01/15	53,499	12/11/15	45,951	Dec '16	1,644,940	53,063	12/20/16	58,445	12/27/16	45,600
Total	19,696,825						Total	21,487,753					
Average	1,641,402	53,968					Ауставе	1,790,646	58,705				
Maximum	1,928,273		Maximum	83,533			Maximum	2,234,772		Maximum	266,270		
Minimum	1,467,234				Minimum	39,934	Minimum	1,605,347				Minimum	45,600

Punp Go on 5 Time MR Detaing Rest Flow 7 AM-11PM 69 Times Respong Total Total Shirtme AND 5.4 Has DAN

Sandy Hook Pump Station Capacity Study - Exhibit 3



NOTES:

- /(1) DISCHARGE FLANGE IS 125# ANSI DRILLING UNLESS NOTED.
- (2) ALL DIMENSIONS ARE IN INCHES UNLESS NOTED.
- (3) 5400'S AND 5400K'S ARE DIMENSIONALLY IDENTICAL.
- (4) RECOMMENDED LOW WATER LEVEL FOR CONTINUOUS OPERATION. W. 774-
- (5) WATER LEVEL MAY BE DRAWN DOWN TO THIS LEVEL FOR SHORT TIME DUTY IN AIR MOTOR RATINGS. DRAW DOWN CAN OCCUR OVER A PERIOD OF ONE HOUR FOR 210 FRAMES AND 15 MINUTES FOR ALL OTHER FRAMES.
- (6) BASES ARE DESIGNED TO HAVE FULL CONTACT WITH GROUT OR A SOLE PLATE GROUTED IN PLACE.
- (7) NOT FOR CONSTRUCTION, INSTALLATION, OR APPLICATION PURPOSES UNLESS CERTIFIED. DIMENSIONS SHOWN MAY VARY DUE TO NORMAL MANUFACTURING TOLERANCES.
- (B) DISTANCE REQUIRED FOR PROPER SUCTION CLEARANCE
 (9) NOTED GUIDE BRACKET USED ONLY ON 10" PUMPS.
 (10) IF RISER PIPE IS NOT SAME SIZE AS THE DISCHARGE ELBOW,
- (10) IF RISER PIPE IS NOT SAME SIZE AS THE DISCHARGE ELBOW, AN ECCENTRIC INCREASER MUST BE USED LIMITED TO TWO SIZES LARGER MAXIMUM.

 UL LISTE

	USTRIES IN	C.			P.O. NO. 97-108	Fairbanks Morse
	NEWTOWN, N	EWTOWN, (т.		SANDY HOOK P. S.	Pump Corporation
PUMP SIZE AND MODEL 4" D5435I	MV /	200 /	165 FT.∕	1766 /	ROTATION CW	BASIC PUMP D5435MV
FAIRBANKS	HP 25 ✓	FRAME 250T	PHASE 3 /	HERTZ 60	VOLTS ENCLOSURE SUB	PULL-UP SUBMERSIBLE FAIRBANKS MORSE MTR
CERTIFIED FOR K4G1	-078230		J HUTCH	INS	FEBUARY 6 1997	DWG 543MS013 REV

Note: Proposed flow (350 gpm) is outside the existing pump curve.

PUMP PERFORMANCE CURVE

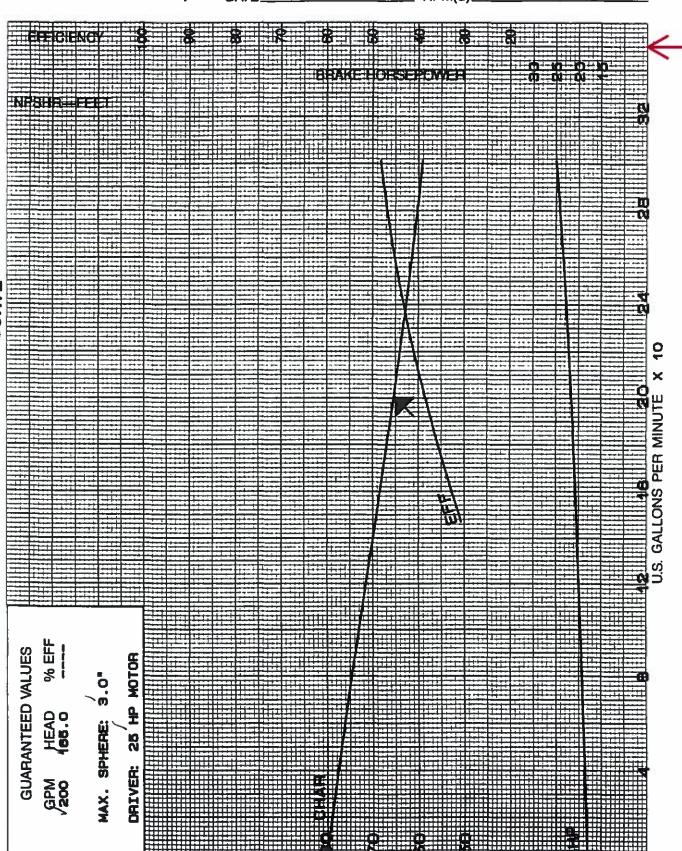
CURVE NO. CK461-078230

Sandy Hook Pump Station Capacity Study - Exhibit

THIS CURVE IS BASED ON ACTUAL TEST PERFORMANCE OF A SIMILAR PUMP. ONLY THE INDICATED POINT(S) IS GUARANTEED.

NO. STAGES_	ONE
REFERENCE	74935
PLOTTED RY	JCM
DATE 2/4/	/97

SIZE-MODEL 4-D5435MV
IMPELLER DES. T4E1E
IMPELLER DIA. 12.60*
RPM(S) 1766



(DIAL YUMY (MUY JAIU)

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Sandy Hook Pump Station Capacity

FUSS&O'NEILL

Exhibit 4

Table 2 - Newtown Flow Calculations

				Infiltration &	Total Peak Hour Flow	Hour Flow	Pump #1	Pump #2
	Average Flow(1)	Average	Peak Factor (2)	Inflow (I/I)	Plus I/I	. М	Capacity (4)	Capacity (4)
Flow Node E (E3, E4)	(pd6)	(dbm)	(pdb)	(pdb)	(pd6)	(mdb)	(mdb)	(md6)
Initial (Table 9-2 POR)	29,970	21	120,180	21,318	141,498	98	215	180
79 Church Hill Rd (225 units)	33,300	23	149,850	(c)	149,850	104		
River Walk (65 units)	9,620	7	43,290	(c)	43,290	30		
Commercial Bldg. (Fabric fire hose)	10,000	7	45,000	(5)	45,000	31		
Total	82,890	58	358,320	21,318	379,638	264	Not OK	Not OK
Based on Actual in 2016	58,705	41	266,270		266,270	185	215	180
79 Church Hill Rd. (225 units)	33,300	23	149,850	(0)	149,850	104		10 mm and 10 mm
River Walk (65 units)	9,620	7	43,290	(5)	43,290	30		
Commercial Bldg. (Fabric fire hose)	10,000	7	45,000	(3)	45,000	31		
Total	111,625	78	504,410		504,410	350	Not OK	Not OK
"							215	180
79 Church Hill Rd. (225 units)	33,300	23	149,850	(3)	149,850	104		
River Walk (65 units)	9,620	7	43,290	(0)	43,290	30		
Commercial Bldg. (Fabric fire hose)	10,000	7	45,000	(1)	45,000	31		
Ultimate (Table 9-3 PDR)-Includes Following								
Developments	341,880	237	1,111,110	596,636	1 707 746	1,186	Not OK	Not OK

Notes:

1- Flow Assumptions: Initial and Final Flow

79 Church Hill Rd (225 units) River Walk (65 units)

Commercial Bldg. (Fabric fire hose)

2- Following peaking factors have been used:

79 Church Hill Rd (225 units) River Walk (65 unils)

Initial Flow

4.01 - Table 9-2 PDR dated 04/20/94 by F&O 4.5 -Table 2-1 of TR-16, 2011 Edition 4.5 -Table 2-1 of TR-16, 2011 Edition 4.5 -Table 2-1 of TR-16, 2011 Edition 3.25 - Table 9-3 PDR dated 04/20/94 by F&O

10,000 - Assumed

148 gpd/unit - Town of Newtown Sewer Use Regulations dated October 29, 2015 148 gpd/unit - Town of Newtown Sewer Use Regulations dated October 29, 2015

- Table 9-2 and 9-3 PDR dated 04/20/94 by F&O

4- Existing pumps can not handle the 165 gpm additional flow from the proposed developments

3- Infiltration & Inflow (III) included in calculated flow

Commercial Bldg. (Fabric fire hose)

Ultimate Flow



1992248.A53 4/20/2017



FUSS & O'NEILL Despite to Defere

Table 3 - Force Main Hydraulic Calculations - Actual Flow

Sandy Hook Pump Station

Description	Value	Units
Pump Rate	350	mdß
C Factor	120	120 Old D.f.
Force Main Length (6")	3,120 feef	feef
Pump Discharge (Ll. Alarm, H)	210.2 feet	feef
Force Main highest Elevation	357.0 feet	feet
Force Main Discharge Elev. at P.S.	226.5	
Force Main Length (4")	40	
Pump Rate		дош
Static Head	147	147 feet

2,524 ft 396 200 3,120 Under Pootatuck River = Church Hill Rd L = To Dry Pit

0.0148

0.0357

2.30 0.30

Gate Valve (Total of 6, one for 4" & 5 for 6")

Swing Check Valve

Branch Cross

Air Release/Vacuum Valve Cleanout

Increaser

Flow Meter

0.30 0.80 0.10

0.0047 0.0124

0.0466 0.0261 0.0186

0.0420

0.90

90 Degree Bend (Standard Radius)

Entrance

90 Degree Bend (Long Radius)

11.5 Degree Bend 45 Degree Bend

Wye Increaser

0.0078

0.0140

0.42

0.0047

× (K + 2g) Force Main (6")

× (K + 2g) P.S. (4")

K Factor

Quantity

Mechanical Friction Head

Type of Fitting

0,0155

0.0016

0.1388

0.1086

Subtotal

Mechanical Friction Head = 0.1086 x (v^2)

Peak Hour (mdB) EdB 23 7 7 23 Avg. Flow 9620 10000 33300 (bdb) Sandy Hook Max Day Flow (Sep. 2016) Commercial Bldg. (Fabric fire hose) 79 Church Hill Rd. (225 units)* **Existing and Future Flow** River Walk (65 units) Proposed Total

Each Units EDUs (Including III) - "Town of Newtown Sewer Use Regulations", October 29, 2015 Peaking Factor Assumptions 188 4.5 185 30 31 350

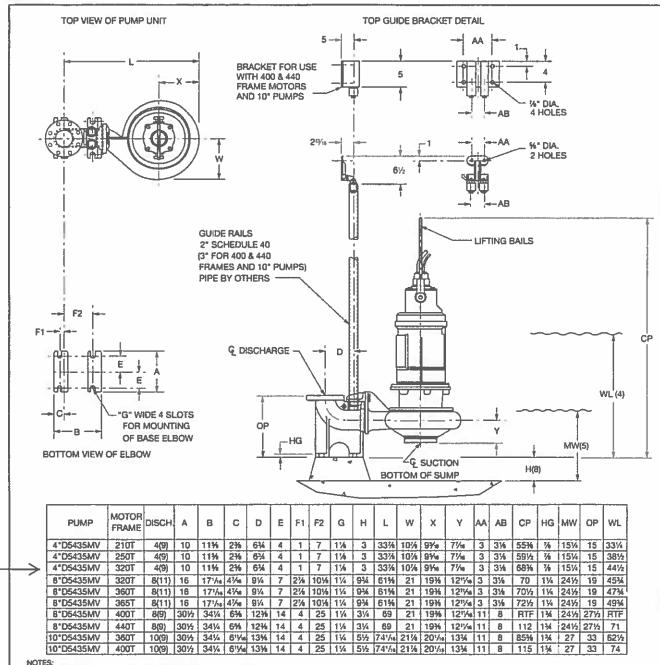
Total Dynamic Head

		12 P.S.	74 FM	
Maximum Pressure	psd	ľ		87
Total Dynamic Head	feet	29	172	200
Static Head	feet	16	131	147
Friction Head (feet)	Mechanical	8.7	2.2	10.9
Friction	Pipe	3.6	38.6	42.4
Velocity	tps	8.94	3.97	
Flow	gpm	320	350	
Pipe Diameter	inches	4	9	Total

(Using Hazen-Williams Equation)

The Hazen-Williams equation with discharge coefficient of C = 120 was used for head loss in Old D.I. Pipe.

D5430 M&W Submersible Solids-Handling Pump Sandy Hook Pump Station Capacity Study Exhibit 5

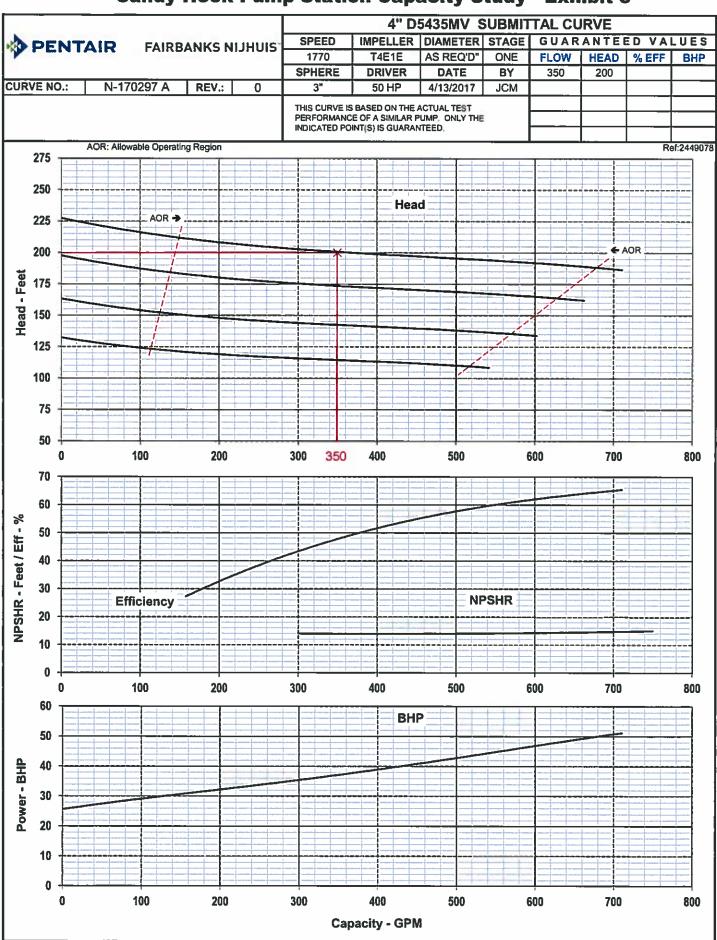


- DISCHARGE FLANGE IS 125# ANSI DRILLING UNLESS NOTED.
- ALL DIMENSIONS ARE IN INCHES UNLESS NOTED. 5400'S AND 5400K'S ARE DIMENSIONALLY IDENTICAL
- RECOMMENDED LOW WATER LEVEL FOR CONTINUOUS OPERATION.
- 210 FRAME AND WATER JACKETED 250 THRU 440 FRAME UNITS CAN OPERATE CONTINUOUSLY AT "MW" WATER LEVEL.
 WATER LEVEL MAY BE DRAWN DOWN TO THIS LEVEL FOR
- SHORT TIME DUTY IN AIR MOTOR RATINGS, DRAW DOWN CAN OCCUR OVER A PERIOD OF 15 MINUTES.
- BASES ARE DESIGNED TO HAVE FULL CONTACT WITH GROUT OR A SOLE PLATE GROUTED IN PLACE.
- NOT FOR CONSTRUCTION, INSTALLATION, OR APPLICATION PURPOSES UNLESS CERTIFIED. DIMENSIONS SHOWN MAY VARY DUE TO NORMAL MANUFACTURING TOLERANCES.
- DISTANCE REQUIRED FOR PROPER SUCTION CLEARANCE.
- IF RISER PIPE IS NOT SAME SIZE AS THE DISCHARGE ELBOW, AN ECCENTRIC INCREASER (9) MUST BE USED LIMITED TO TWO SIZES LARGER MAXIMUM.
- IF FUTURE CONSIDERATIONS REQUIRE CHANGING TO A 400 OR 440 FRAME MOTOR, THE 400 OR 440 FRAME DIMENSIONS SHOULD BE USED.
- IF RISER PIPE IS NOT SAME SIZE AS THE DISCHARGE ELBOW, AN ECCENTRIC INCREASER MUST BE USED LIMITED TO ONE SIZE LARGER MAXIMUM. UL LISTED

ISO-9001 CERTIFIED

CERTIFIED FOR			CERTIFIED BY		DATE		543MS013 No 5
							FAIRBANKS MORSE MTR
MOTOR	HP	FRAME	PHUSE	HERTZ	VOLTS	ENCLOSURE	PULL-UP SUBMERSIBLE
		- · · ·		1			D5435MV
PUMP SIZE AND MOD	91	ОРМ	(TDH	RPM	ROTATION		BASIC PUMP
JOB MANE					TAG NAME		
CUSTOMER					P.O. NO.		Fairbanks Morse Pump
							CSA CERTIFIED (THRU 365 PRAME)

Sandy Hook Pump Station Capacity Study - Exhibit 5



Attachment B

Analysis of Available Sewer Capacity – Preliminary 6/8/17

	Current Sewer Plan	Changes	Adjusted Results
Metered Flow*	267,000	+5,000	272,000
Allocated**	31,630	- 9,100	22,530
Reserve	9,960		9,960
Unallocated***	* 23,410	+4,100	27,510

There is no change in allocation for any pickup from projected corrections from improvements in reducing 1&I.

- *This is the average metered flow since 11/14.
- ** Sandy Hook School, Lexington Gardens and The Boulevard are removed from this line.
- *** This line is adjusted for the changes above.

At the current time we have the following potential projects for which capacity may be required:

The Boulevard: 75 units of assisted living – 5,400 gpd.

Fabric Fire Hose: 2,000 gpd.

79 Church Hill: 225 units – 28,125 gpd.

Total potential request of 35,525 gpd.



CT NEWTOWN WWTF

5/1/2017 to 5/31/2017

m TN % Ren			91.3							92.0							89.2								88.1		1				90.0		90.1	450.5	88
TSS % Rem			99.5							2 66							99.5								9.66						9.66	1	9.66	497.9	5 00
BOD % Rem	%		1'66							99.3							99.2								2'66						2'66		99.2	495.9	1 66
Eff Alk mg/l	mg/L		0/																														70	0/	70
Eff TN lbs	lbs/day		8.11							13.8							9'51								8.91						11.1		13.8	69.1	
Eff TN mg/l	mg/L	,	3.0							2.5							3.4								4.1						2.9		3.2	6'51	٦,٥
D mg/ Eff TSS mg/l Eff TN	mg/L		9.0							0.5							6'0								0.7						0.7		0.7	3.4	0.5
Eff BOD mg/l	mg/L		1.3				7			1.0							1.2								1.2						1.1		1.2	5.8	0.1
Inf Alk	-		150							1																							150	150	150
Inf TN Lbs	lbs/day		129							161							132								144		1,				86		132.800	664.064	121 00
Inf TSS Lbs	lbs/day		200							859							707								731						535		666.400	3,332,666	700 650
Inf TSS mg/l	mg/L		133							167			1				891								175			8 -			158		160	801	133
Inf BOD Lbs	_		521	1						726			1				602								614						481		589	2,943	187
OD mg/	mg/L		6£1							141							143								147						142		142	712	130
Inf Q Total Inf B	г	0.471	0.450	0.462	0.439	8/9.0	0.678	8/9'0	0.658	0.617	0.561	0.512	0.579	0.579	0.579	0.546	0.505	0.442	0.468	0.507	0.507	0.507	0.515	0.486	0.501	0.527	0.465	0.465	0.465	0.465	0.406	0.426	0.521	16.145	0.406
	Date	05/01/17	05/02/17	05/03/17	05/04/17	05/05/17	21/90/50	05/07/17	05/08/17	05/09/17	05/10/17	21/11/50	05/12/17	05/13/17	05/14/17	05/15/17	05/16/17	05/17/17	05/18/17	05/19/17	05/20/17	05/21/17	05/22/17	05/23/17	05/24/17	05/25/17	05/26/17	05/27/17	05/28/17	05/29/17	05/30/17	05/31/17	Average	Total	Minimim

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Newtown Water and Sewer Monthly Flow Summary

May	Sandy Hook PS	Taunton Lake PS	Baldwin PS	Hawleyville PS	Hanover PS
2017	Flow Total (Gal)				
1	72,957	17,189	3,708	28,030	4,754
2	76,614	17,432	2,870	29,179	5,013
m	75,611	16,397	2,997	30,201	5,303
4	72,071	15,597	2,986	28,980	5,246
2	81,588	19,242	4,283	32,638	5,602
9	78,527	26,249	4,214	28,202	6,397
7	73,649	26,430	4,209	28,831	6,049
00	77,499	22,026	3,463	30,451	4,965
6	72,675	20,664	3,798	30,651	5,166
10	73,859	19,564	3,284	31,561	4,762
11	76,867	19,671	3,220	29,895	4,601
12	72,946	19,035	3,532	30,669	4,625
13	71,737	21,089	3,941	32,524	6,035
14	73,310	28,922	4,723	28,281	6,262
15	76,144	22,748	3,249	31,939	5,287
16	76,986	21,485	3,378	31,703	4,469
17	74,381	20,473	3,025	30,257	5,321
18	71,363	18,697	1,615	30,669	6,129
19	66,820	18,929	3,093	30,002	8,916
20	61,935	21,223	2,790	29,905	9,745
21	59,934	18,904	3,476	31,732	10,168
22	68,037	17,921	3,493	33,730	9,791
23	66,330	18,707	2,794	31,371	9,791
24	64,754	19,240	3,002	34,483	9,082
25	67,536	18,951	3,015	31,336	10,247
26	73,846	22,110	4,274	32,507	10,064
27	66,128	20,629	3,713	31,947	11,514
28	968'65	19,750	3,576	29,894	10,645
29	58,843	21,255	3,938	32,714	10,645
30	99,368	19,003	3,351	28,436	8,288
31	66,668	18,893	3,001	30,161	10,169
Min	58,843	15,597	1,615	28,030	4,469
Max	81,588	28,922	4,723	34,483	11,514
Ave	70,819	20,272	3,420	30,738	7,260
Total	2,195,380	628,429	106,012	952,880	225,050

Newtown Water and Sewer Monthly Flow Summary

May	Pump Stations	Fairfield Hills	Combined Remote	WWTP Influent	Gravity Flow	Rain E	Rain Fall (inch)
2017	Flow Total (Gal)	Annual	Daily				
1	126,639	176,675	303,314	479,961	126,639	0.00	0.00
2	131,108	171,159	302,266	447,695	131,108	0.00	0.00
m	130,511	168,125	298,636	459,343	130,511	0.00	0.00
4	124,880	162,720	287,600	442,286	124,880	0.00	0.00
Ŋ	143,353	219,196	362,549	593,665	143,353	0.00	0.00
9	143,589	251,515	395,105	657,183	143,589	0.00	0.00
7	139,169	281,332	420,501	703,917	139,169	0.00	0.00
80	138,403	231,797	370,199	661,995	138,403	0.00	0.00
6	132,953	198,797	331,749	632,432	132,953	0.00	0.00
10	133,030	198,128	331,158	563,736	133,030	0.00	0.00
11	134,254	189,443	323,696	528,200	134,254	0.00	0.00
12	130,808	189,383	320,191	522,330	130,808	0.00	0.00
13	135,326	204,055	339,381	567,974	135,326	0.00	0.00
14	141,499	226,808	368,307	620,543	141,499	0.00	0.00
15	139,368	208,404	347,773	560,661	139,368	0.00	0.00
16	138,021	198,658	336,679	507,868	138,021	0.00	0.00
17	133,456	187,920	321,377	462,208	133,456	0.00	0.00
18	128,473	176,653	305,126	454,188	128,473	0.00	00.0
19	127,760	178,759	306,519	498,053	127,760	0.00	0.00
20	125,598	169,183	294,780	501,283	125,598	0.00	0.00
21	124,213	185,659	309,873	498,275	124,213	0.00	0.00
22	132,973	202,869	335,843	539,834	132,973	0.09	60.0
23	128,993	173,895	302,888	475,318	128,993	0.09	0.00
24	130,561	170,307	300,868	472,948	130,561	0.10	0.01
25	131,085	179,517	310,603	493,482	131,085	0.41	0.31
56	142,801	224,984	367,785	540,885	142,801	1.02	0.61
27	133,931	184,727	318,658	464,563	133,931	1.02	0.00
28	123,261	182,443	305,704	434,552	123,261	1.02	0.00
29	127,395	173,429	300,825	453,668	127,395	1.07	0.05
30	125,446	179,388	304,834	449,472	125,446	1.07	00:00
31	128,892	173,783	302,675	400,313	128,892	1.15	0.08
Min	123,261	162,720	287,600	400,313	123,261		0.00
Max	143,589	281,332	420,501	703,917	143,589		0.61
Ave	132,508	194,184	326,692	518,994	132,508		0.04
Total	4,107,751	6,019,711	10,127,462	16,088,829	4,107,751		1.15

Newtown Water and Sewer Monthly Flow Summary

May	Well House #3	Well House #7	Booster Pumps
2017	Flow Total (Gal)	Flow Total (Gal)	Flow Total (Gal)
1	0	136,811	138,036
7	95	128,559	129,687
m	0	142,183	143,410
4	0	134,825	136,061
S.	0	131,024	132,214
9	0	138,826	143,250
7	0	208,863	207,533
00	0	159,797	161,627
on.	0	135,295	136,475
10	0	133,537	134,779
11	0	124,988	126,233
12	0	137,067	140,558
13	0	121,236	120,678
14	0	132,864	134,223
15	0	128,824	132,908
16	0	137,477	135,767
17	0	145,637	146,987
18	0	126,018	127,106
19	0	141,924	143,302
20	0	124,477	125,808
21	0	152,450	153,837
22	0	159,889	161,116
23	0	139,270	140,770
24	0	139,025	140,136
25	0	140,268	141,816
56	0	141,661	142,916
27	0	133,553	134,630
28	0	131,781	133,046
59	0	136,809	138,501
30	0	135,113	136,405
31	0	210,630	211,084
Min	0	121,236	120,678
Max	95	210,630	211,084
Ave	m	141,635	142,932
Total	95	4,390,681	4,430,899